

Geotechnical Engineering Failures and Lessons Learned

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Outline

- ▶ What is a failure?
- ▶ What factors lead to failures?
- ▶ Case Histories
- ▶ Questions

What is a failure?

- ▶ Constructed work does not meet legal requirements (i.e. building codes, location restrictions, standard of care)
- ▶ Does not serve intended purpose or meet client criteria
- ▶ Is not cost-effective
- ▶ Does not reach design life

What factors lead to failures?

- ▶ Limited Budgets
- ▶ Technical errors
- ▶ Lack of sufficient information
- ▶ Poor communication
- ▶ Lack of adequate supervision or testing during construction
- ▶ Unanticipated conditions encountered during construction
- ▶ Politics
- ▶ Poor maintenance
- ▶ Court settlements promote confidentiality, so mistakes are often repeated rather than learned from

Engineering Failures – Case Histories



Menard Hall
Fargo, ND

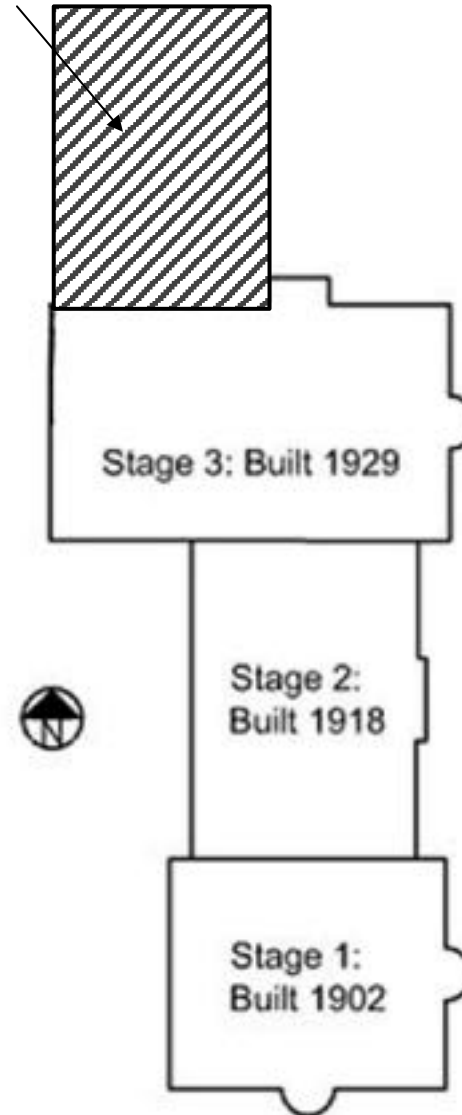
December 2009



Building Addition

- Three levels above grade, two levels below grade
- Existing structure on shallow spread footing foundations close to ground surface
- Subsurface materials – Lake Agassiz clay

Proposed Addition

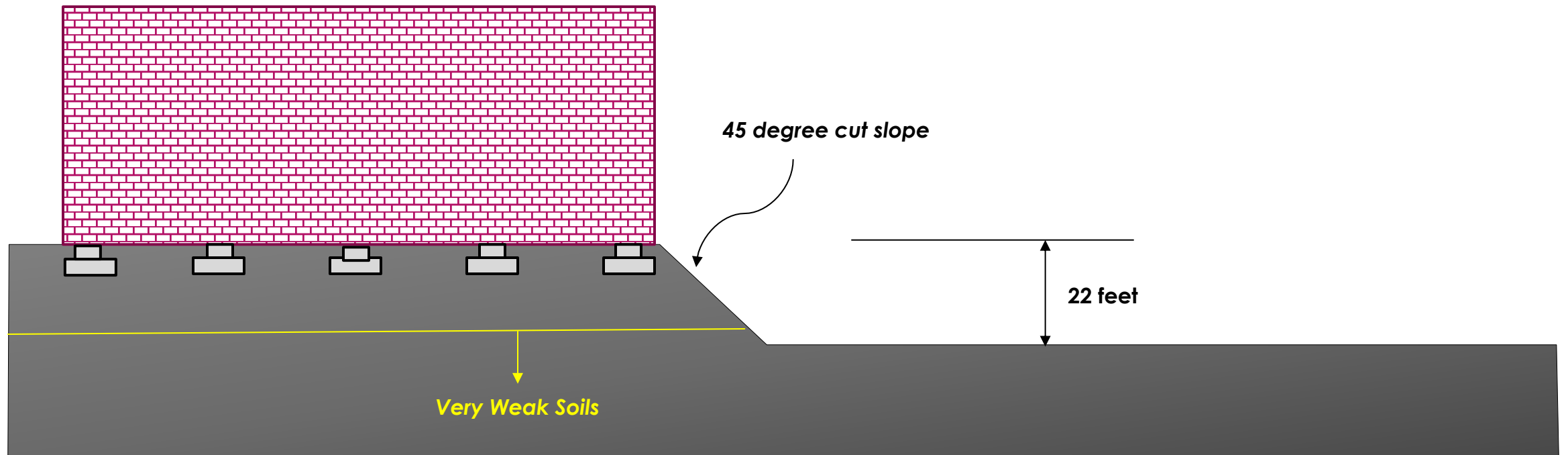


Building Collapse

- ▶ Collapse of 1929 section of building shortly after midnight on December 27, 2009.
- ▶ No one injured.



Building Addition



Investigation

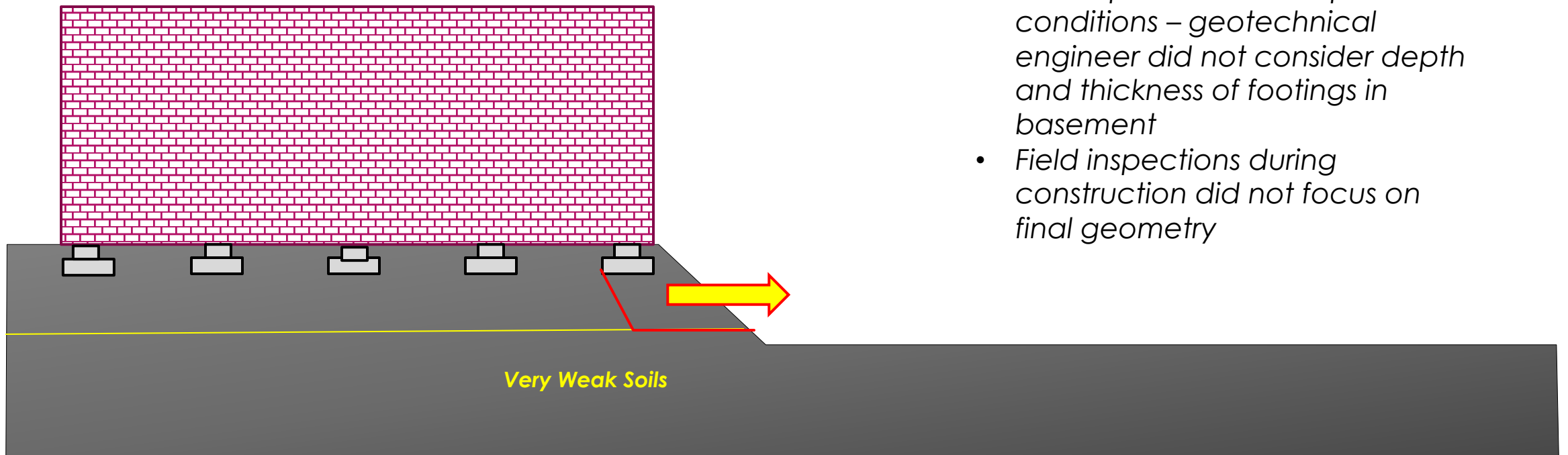


Displacement of Foundation Wall – Predominantly Lateral



Failure Surface in Clay Below Footing

Conclusions



- Cut slope was too steep for conditions – geotechnical engineer did not consider depth and thickness of footings in basement
- Field inspections during construction did not focus on final geometry

Demolition and Reconstruction



Lessons Learned

- ▶ Consider final configuration of cut slopes
- ▶ Be conservative in selection of material properties – use lower bound strengths, when consequences of failure are high
- ▶ Use excavation bracing systems to protect critical structures – this is not the best place to save on construction costs
- ▶ Install monitoring devices and take readings frequently during construction

Marriott
Courtyard Hotel
14th Street
Atlanta, Georgia

June 1993



Marriot Courtyard

- ▶ A depression was noted in the parking lot near a storm sewer manhole in mid-June 1993
- ▶ Small area roped off to protect public
- ▶ Studies of the cause of the depression were planned and scheduled

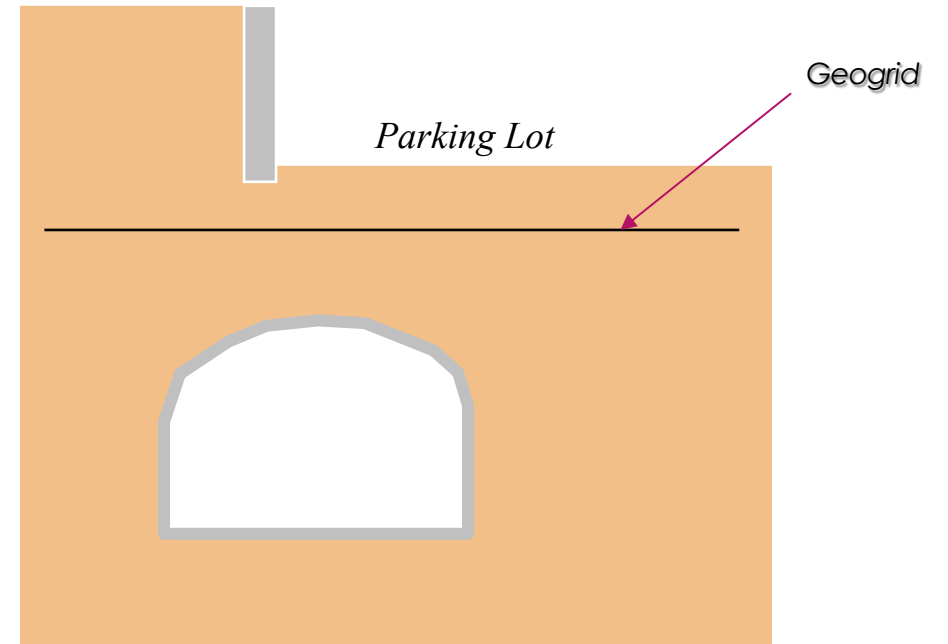
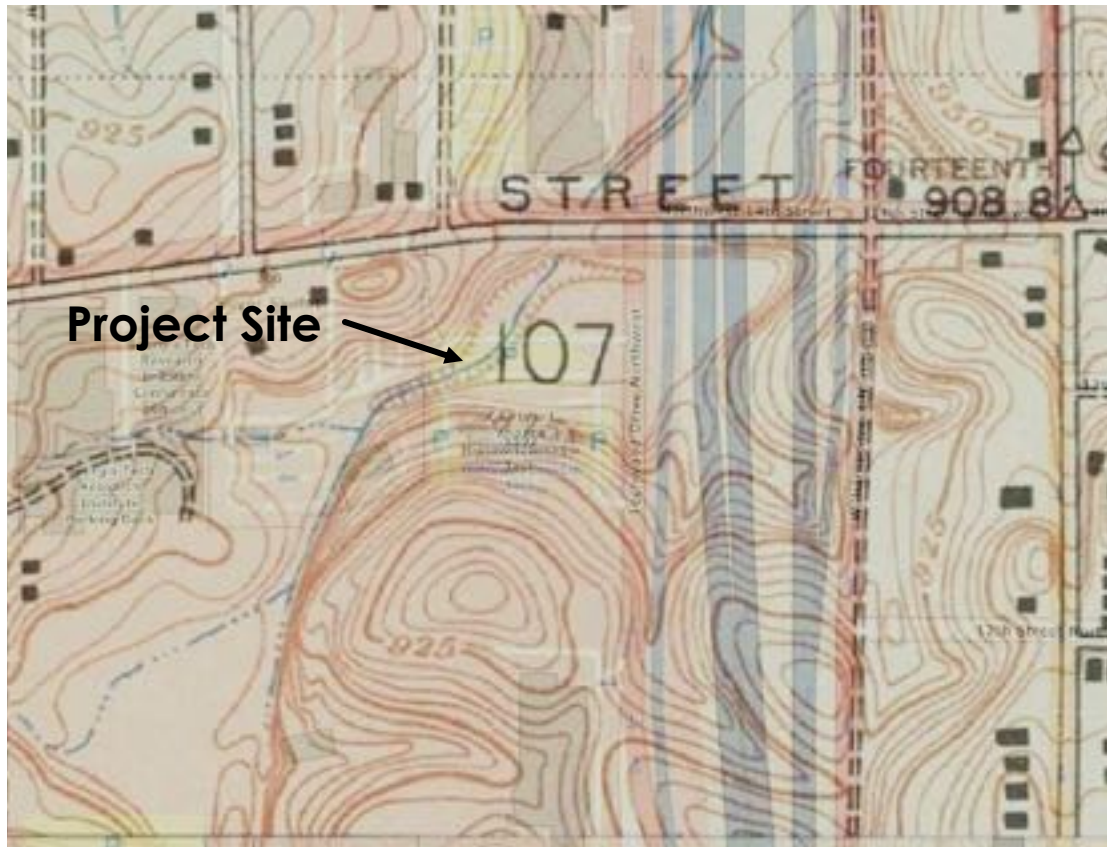


Marriot Courtyard

- ▶ The next day on June 14th, Atlanta experienced a 50-year storm event
- ▶ The depression in the parking lot widened and a large puddle formed causing a car to stall when entering parking lot
- ▶ The car's driver entered the hotel and requested assistance from an employee.
- ▶ Upon reaching the car, a sinkhole nearly 200 feet in diameter dropped out, swallowing both individuals and three cars.



Site History



Sewer Line Configuration

Lessons Learned

- ▶ Communication failed at several points:
 - ▶ Developer did not inform operator of the presence of geogrid or its purpose
 - ▶ Inspector did not communicate extent or urgency of the problem to the hotel operator
- ▶ In some instances, mitigation techniques can mask a problem and increase the consequences of failure

Ritz Carlton
Kapalua
Hawaii

1993



Hotel Configuration



Ritz Carlton

- ▶ Within 2 months of opening, slab on grade floors sagged and sounded hollow in public areas of the hotel
- ▶ Voids up to 12 inches in height found under slab “on grade” floors



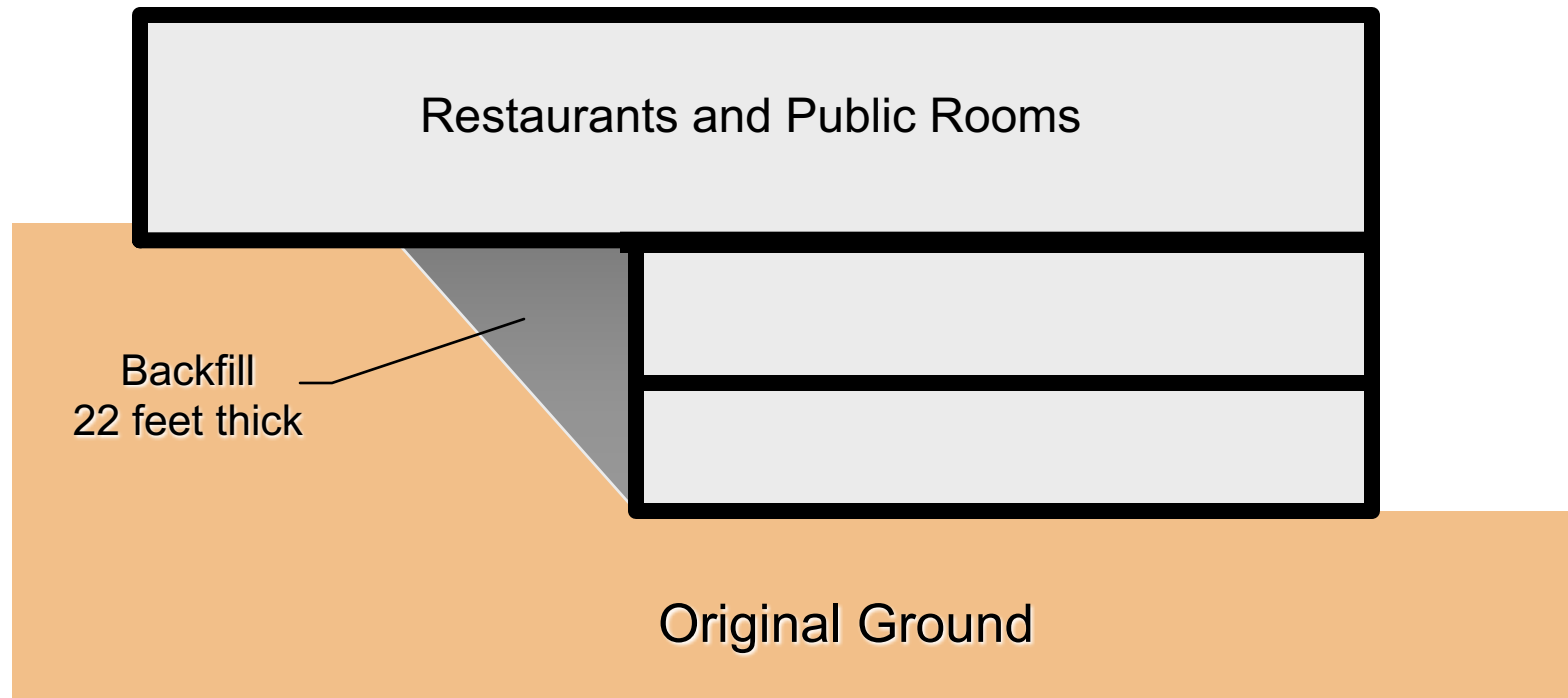
Investigation

- ▶ Review history of construction
- ▶ Testing of subgrade soils
- ▶ GPR testing of floor slabs

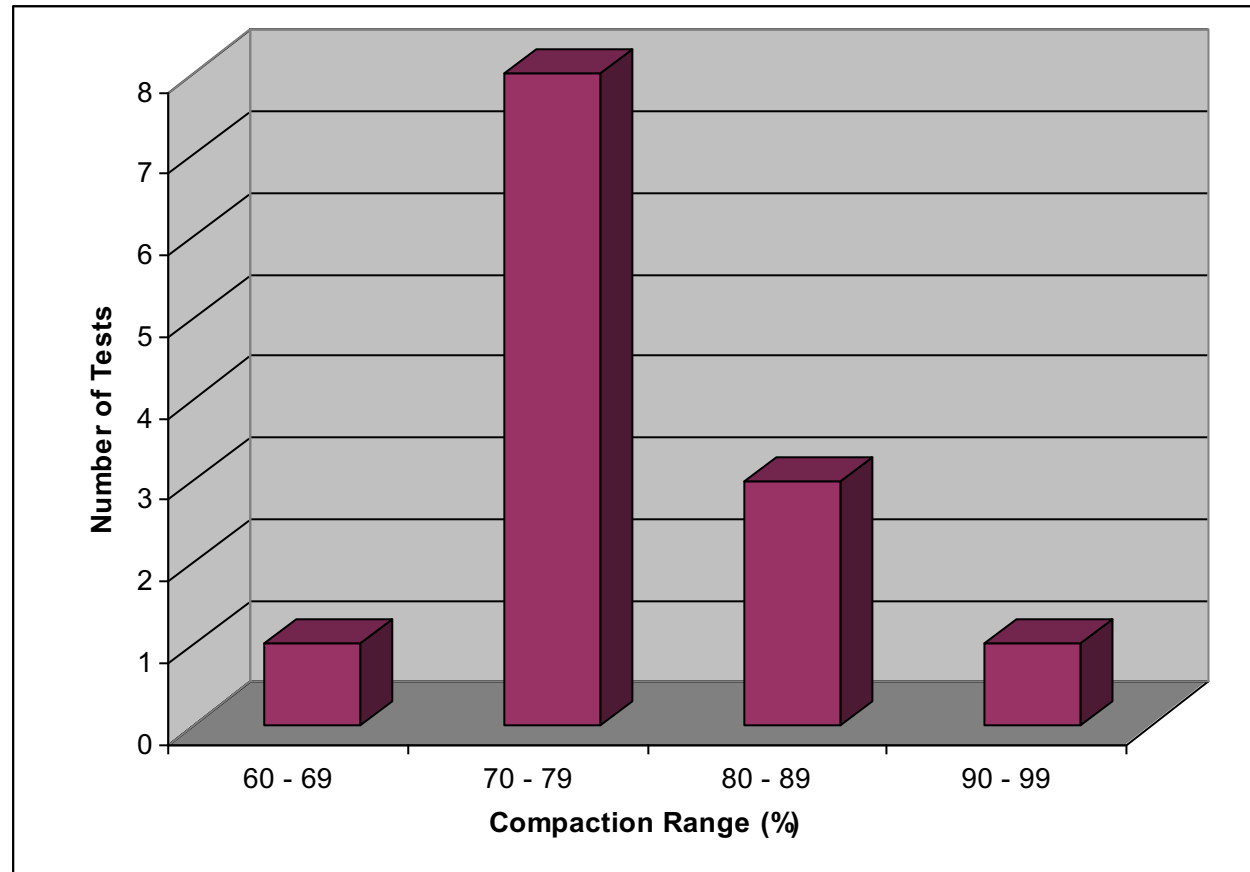
Construction

- ▶ Initial hotel construction encountered ancient burial ground
- ▶ Hotel site was moved upslope
- ▶ Developer cut testing budget to offset cost over-runs

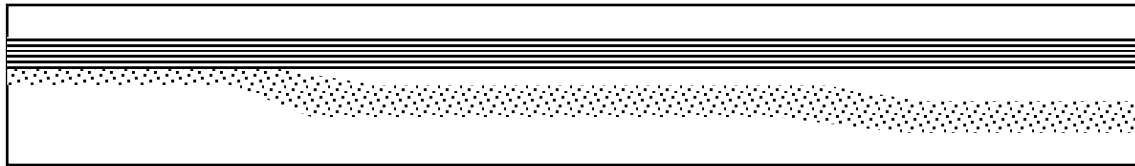
Cross-section through Hotel



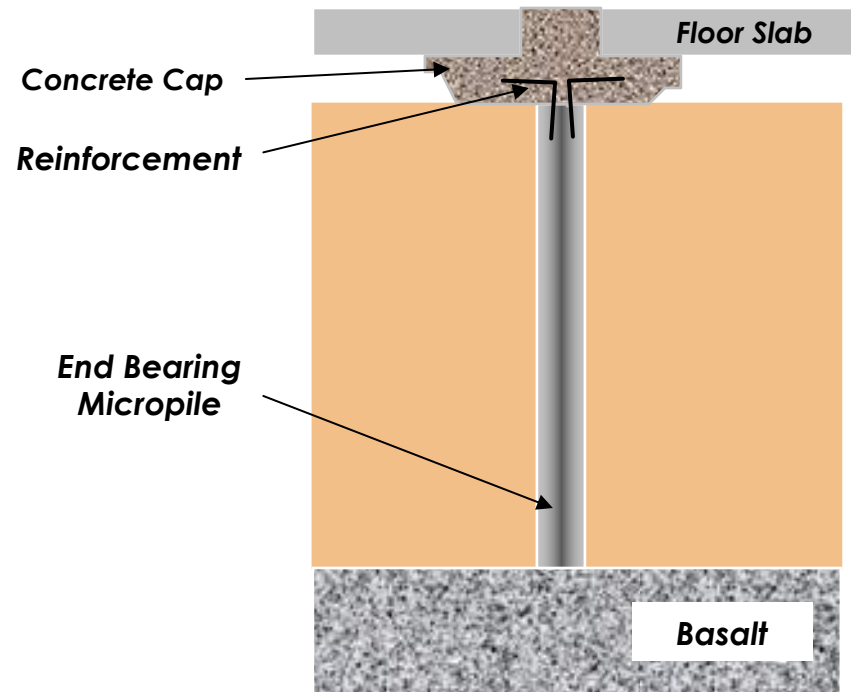
Results of Compaction Tests - Backfill



GPR Test Results



Remediation – Over 1000 Micropiles



Outcome

- ▶ Over \$7 million in direct remediation costs
- ▶ Hotel restaurants and meeting rooms shut down for several months
- ▶ The hotel is now notorious for having experienced settlement – loss of reputation

Lessons Learned

- ▶ Construction testing and inspection should not be cut to compensate for budget over-runs
- ▶ Construction testing is relatively inexpensive when compared with consequences of poor workmanship
- ▶ Cost of repairs are only a fraction of the total cost of failures.

Overlook Village
Shopping Center
Asheville, NC

1978



Site Configuration

- ▶ Significant cuts and fills dictated use of retaining walls near property boundaries
- ▶ Crib wall selected due to low relative cost

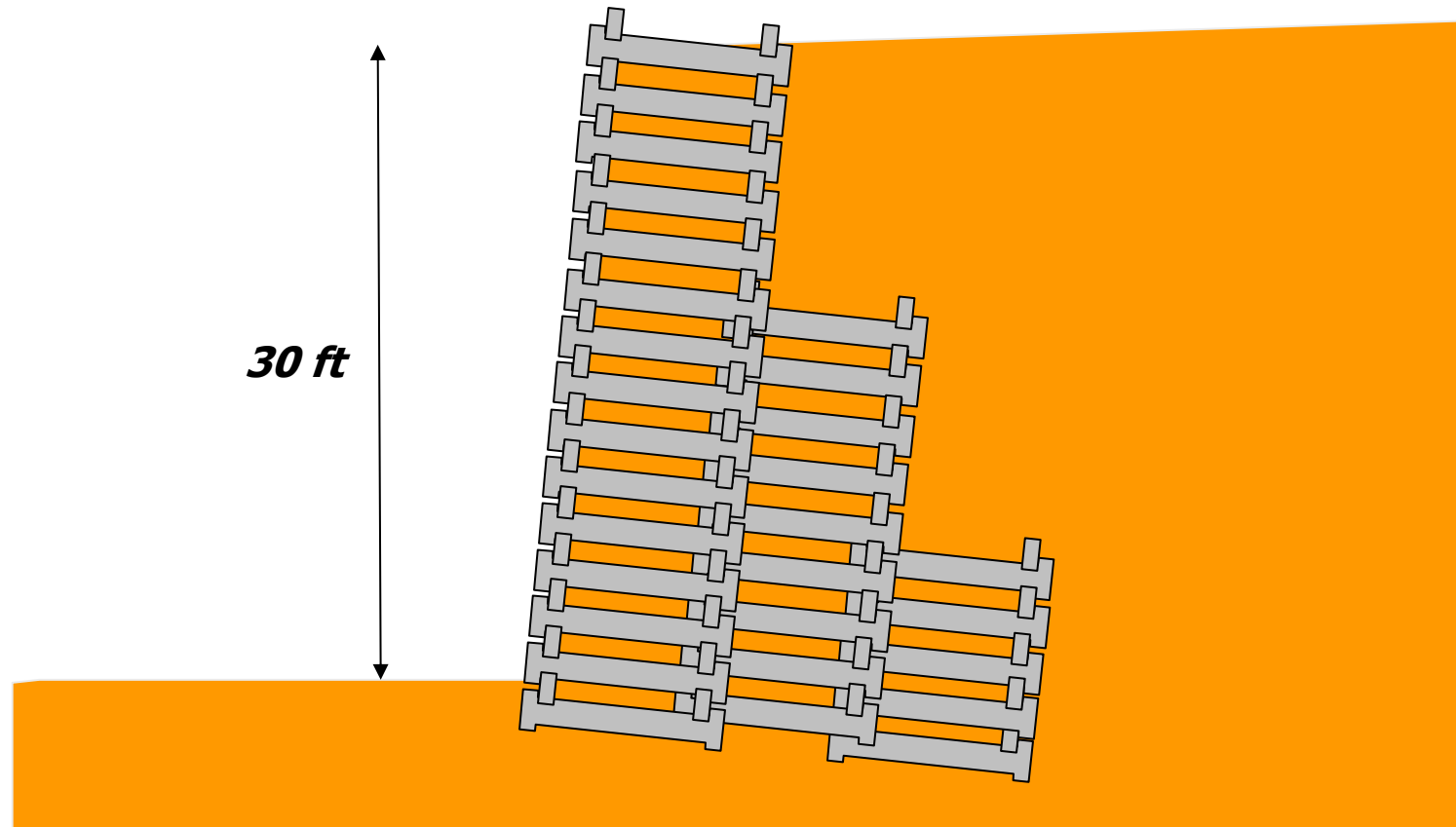


Crib Wall Failure

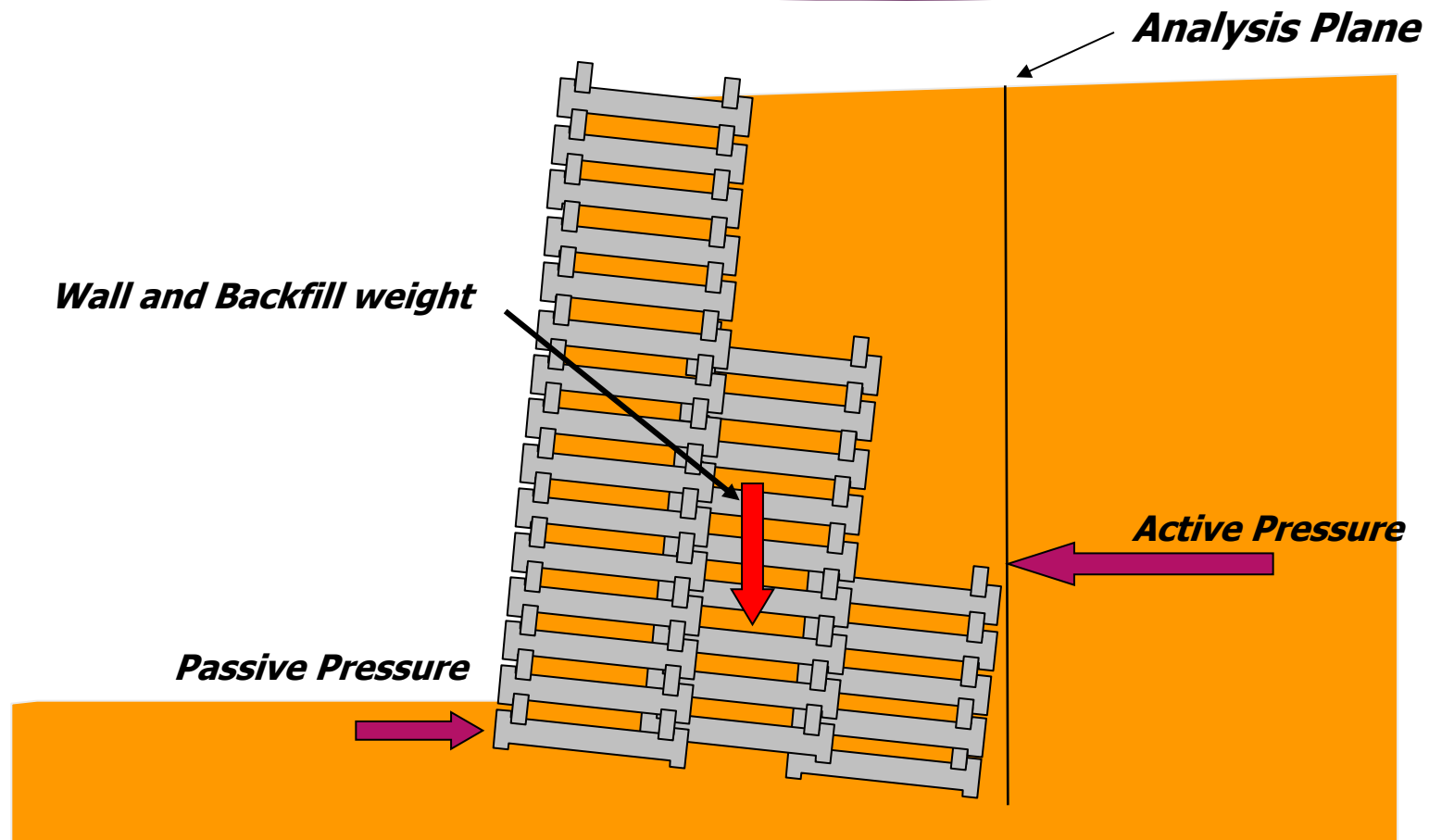
- ▶ Within 6 months of construction, crib wall failed
- ▶ Threatened parking lot of adjacent shopping center



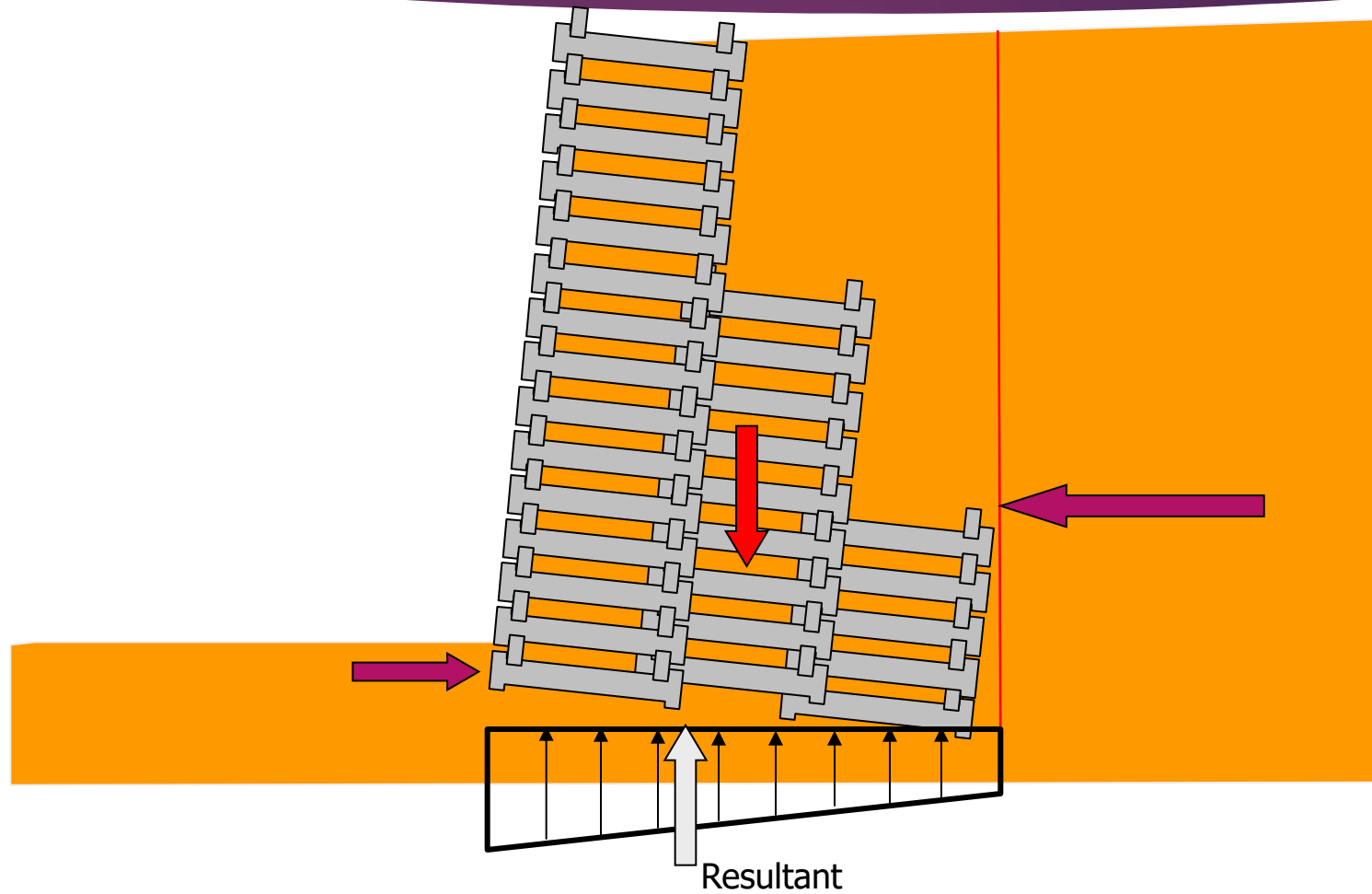
Wall Configuration



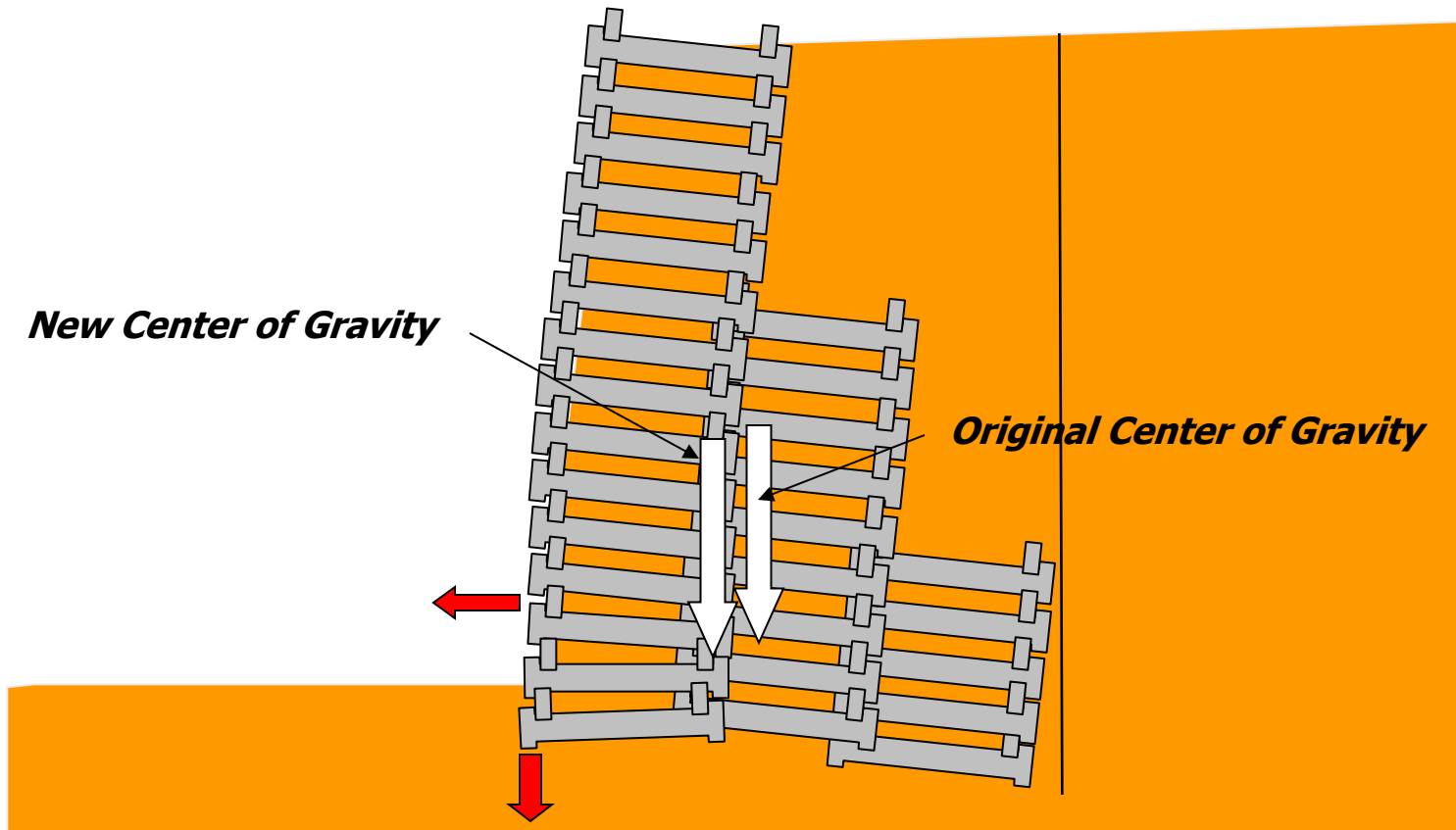
Stability Analysis



Gravity Wall Analysis Results



Crib Walls are not Rigid!



Analysis

- ▶ Walls over 25 feet in height were found to settle differentially leading to a progressive failure over time.
- ▶ More massive walls were required to shift the resultant of pressures to the center of the wall base to avoid differential settlement.

Reconstructed Wall As It Looks Today



Lesson Learned

- ▶ Computer programs are only as useful and accurate as the assumptions made and data inputs used

Confidential Building – Texas Gulf Coast

2008

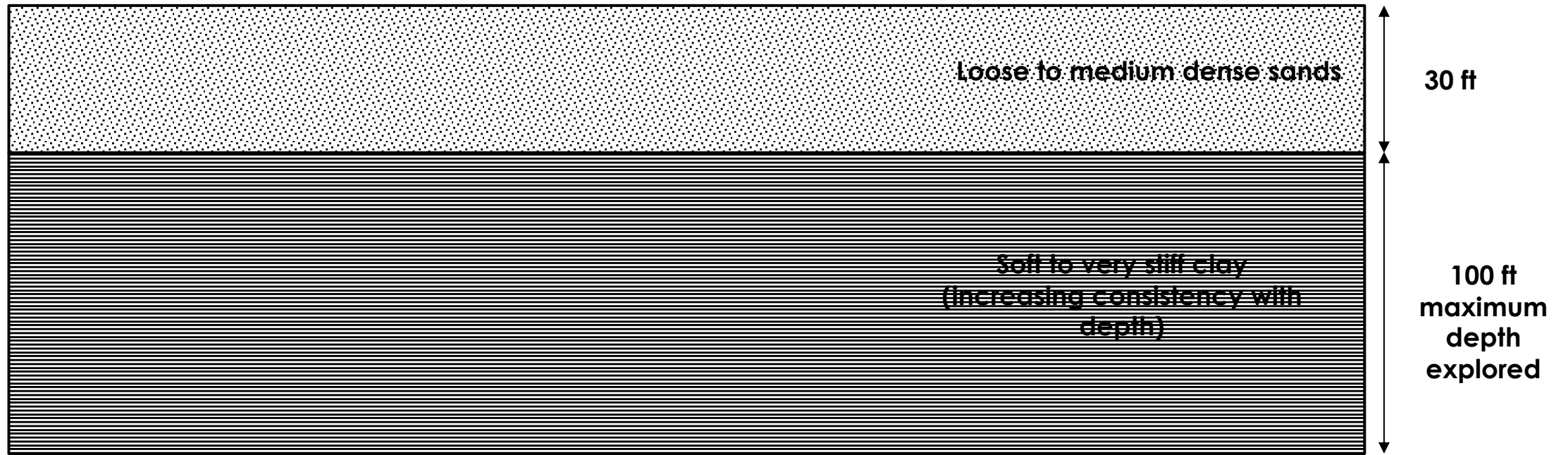


Confidential Building

- ▶ 31 story condominium
- ▶ Began to settle significantly when it reached floor 17
- ▶ Over 16 inches of differential settlement caused massive damage to columns and structural frame, particularly at the interface between the high and low-rise sections

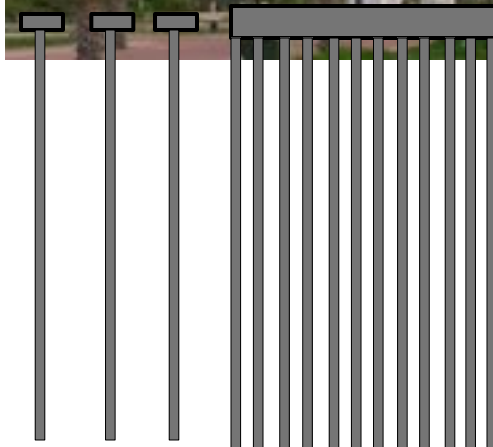


Subsurface Profile



Structural Configuration

- Mat foundation supported by 100 foot long, 16 inch diameter augercast piles (125-ton capacity) under tower
- Pile caps supported by 16 inch diameter augercast piles in parking deck
- Structure post-tensioned



Analysis

- Blow counts in the test borings were not corrected for depth. What appeared to be 24 blow per foot material was closer to 10 blows per foot after correction.
- Design did not consider the group effects of 500 piles under the tower.
- The low and high-rise sections were structurally connected and experienced 16 inches of differential movement over 1 bay.
- While tower settled relatively uniformly, and was structurally sound, it could not be separated from the parking deck due to the post-tensioning cables.

Final Outcome



Lessons Learned

- ▶ Pile groups do not perform the same as individual piles – consider group effects
- ▶ Isolate high rise from low rise sections of buildings as they will experience different magnitudes of settlement

Black Thunder Mine - Wyoming

2009



Conveyor System Required to Transport Coal to Railway

- ▶ Required use of massive crane (over 300 feet in height)
- ▶ Crane supported by timber mats bearing on subgrade soils
- ▶ Very low tolerance to out of level operation



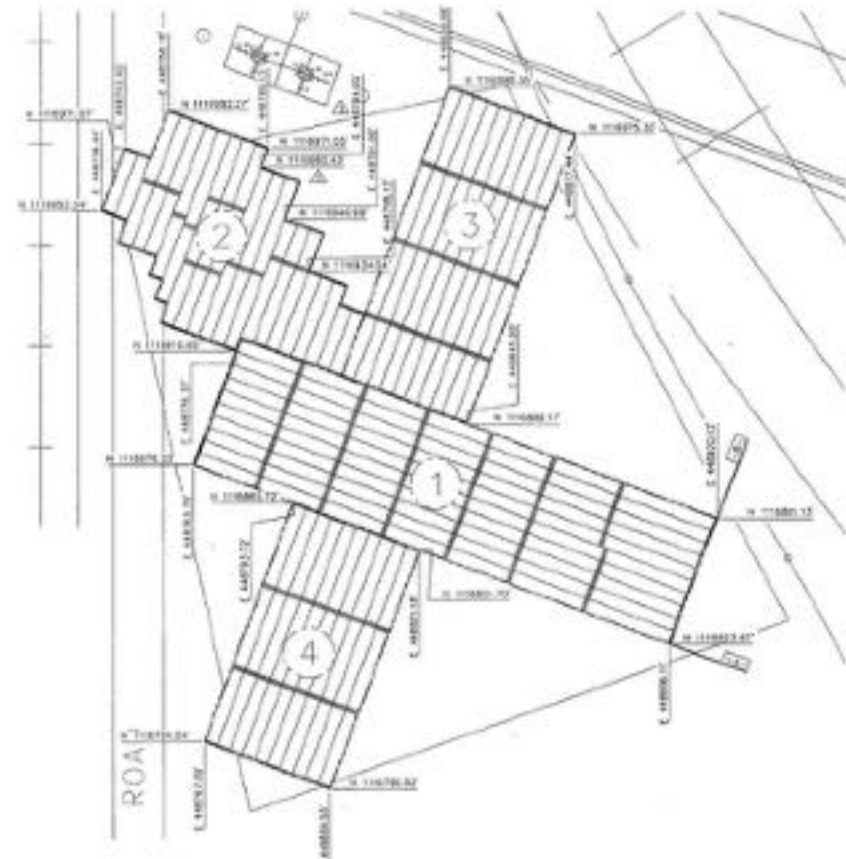
Crane Collapse

- ▶ Crane collapsed while installing a 1/2 million pound section of conveyor system
- ▶ Three workers injured
- ▶ Rail line blocked
- ▶ Total loss of \$3 million crane



Timber Mat

- ▶ Mat placed on sandy silt soil – subgrade inspected by project geotechnical engineer and judged to be suitable
- ▶ Mat situated in low area between two sets of railroad tracks
- ▶ 4 inches of rain recorded in days leading up to failure – water ponded around timber matting



Timber Mat After Collapse



Timber Mat Settlement



Lessons Learned

- ▶ Engineers should consider support capacities of soil under all environmental conditions
- ▶ Contractors should react to obvious and observable changes in conditions and should seek support from qualified engineers

Confluence
Tower
Denver,
Colorado

2016

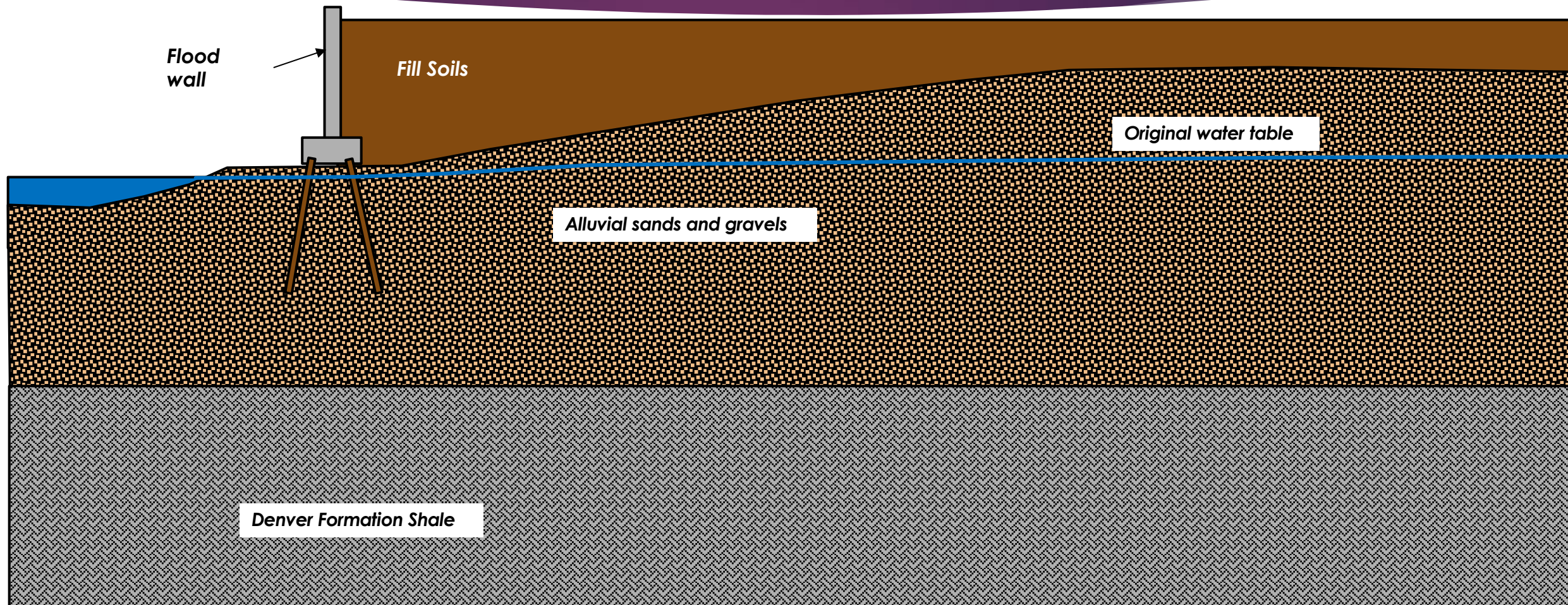


Confluence Tower

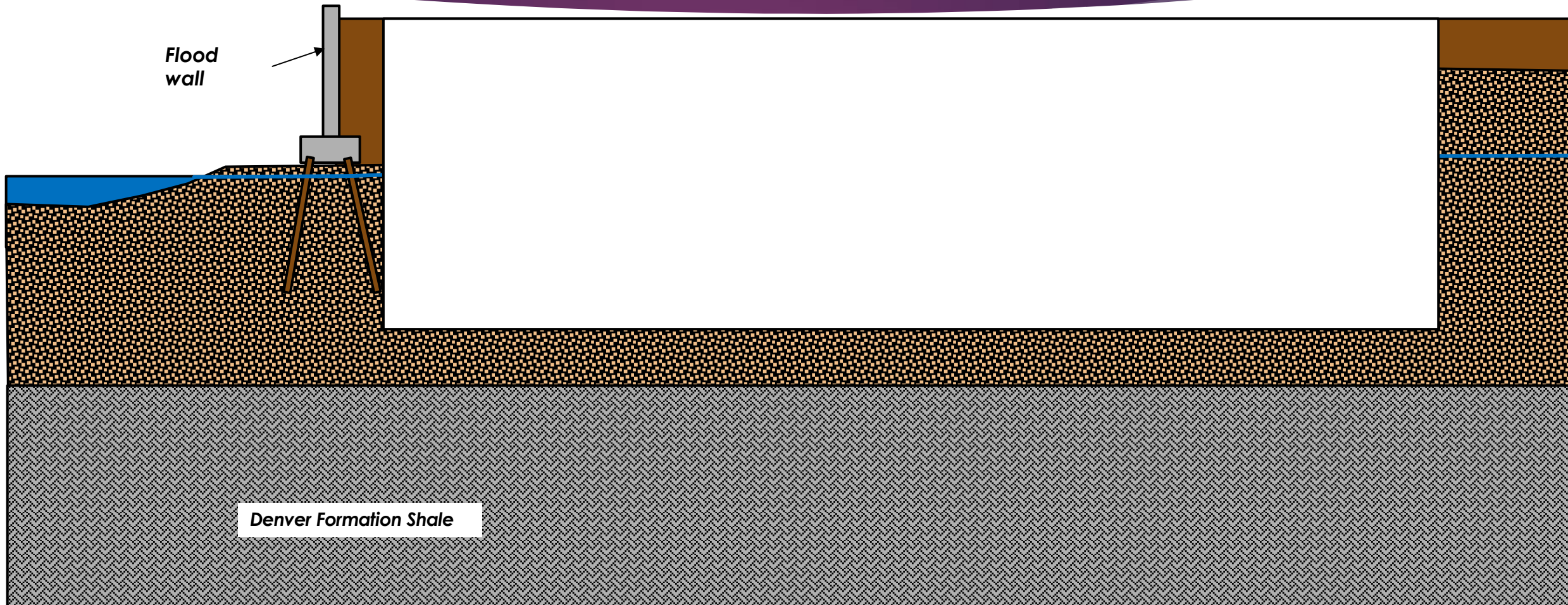
- 3 below grade levels adjacent to Cherry Creek creek and the South Platte River
- 40 feet of coarse sand over weak shale bedrock
- 34 above grade levels



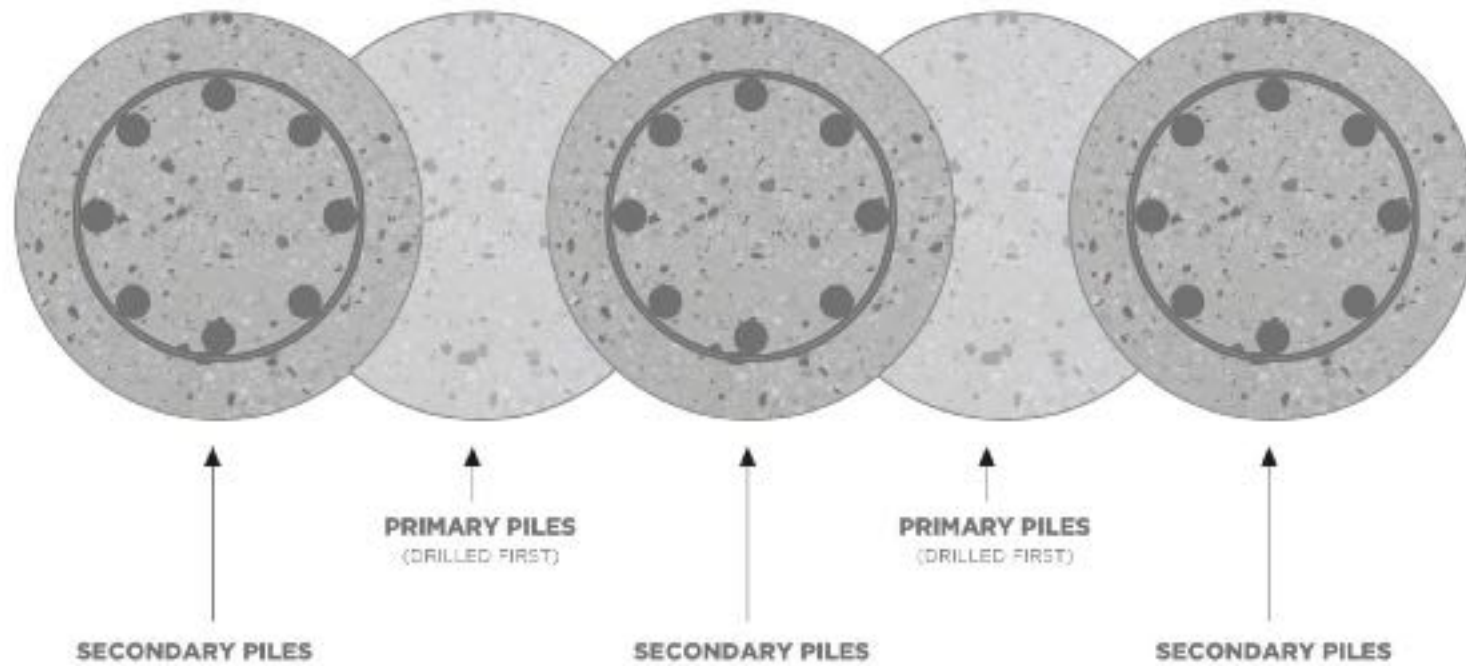
Profile Through Site



Profile Through Site - Construction



Secant Pile Wall

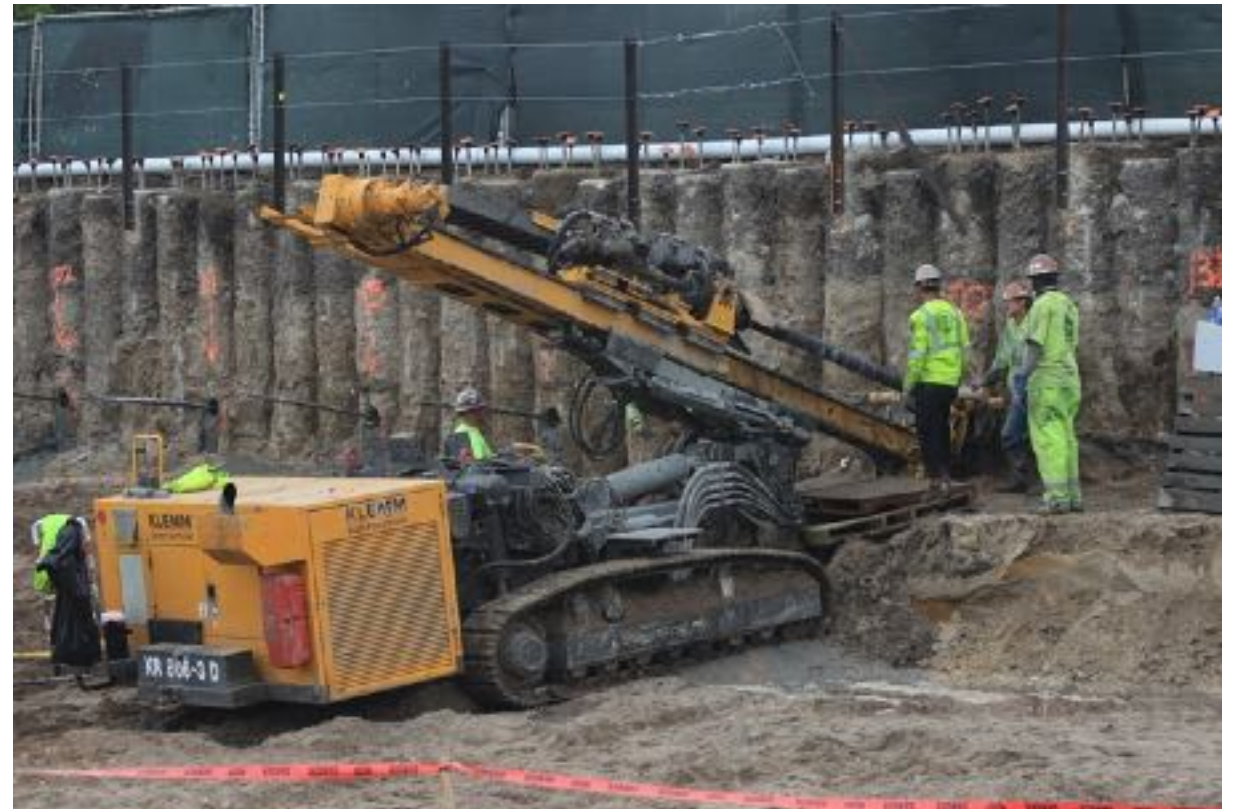


Secant Pile Walls

- ▶ Overlapping CFA piles 3 feet in diameter to create impervious barrier
- ▶ Socketed 5 feet into bedrock



Excavation



Misalignment



**Misaligned
Piles**



**Old Timber
Piles**

Resulting Problem

**Sinkhole
Exterior to the
Excavation**



Causes of Problems

- CFA Rig used multiple flights with non-rigid connections
- Instrumentation in cab only measured inclination of mast, not the auger during drilling



After the Seals.....



**Tying Steel
for the Shear
Wall Mat**



**Topping Out
December
2016**

Lessons Learned

- ▶ Use proper equipment for conditions – cased piers would have been more appropriate for this project
- ▶ In top down construction, it is not always possible to detect problems, even with an experienced inspector

Questions

